



Segunda Ronda de Trabajo Perú Japón
Sobre Mitigación de Desastres

Proposal Method Earthquake Response Diagnosis on Masonry Buildings

Dr. Carlos Zavala
Msc. Patricia Gibu
Ing. Rafael Salinas

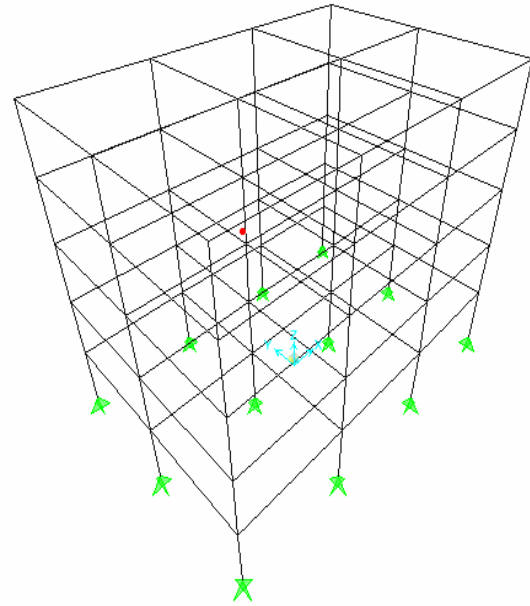


**CENTRO PERUANO JAPONES DE INVESTIGACIONES
SISMICAS Y MITIGACION DE DESASTRES - CISMID**

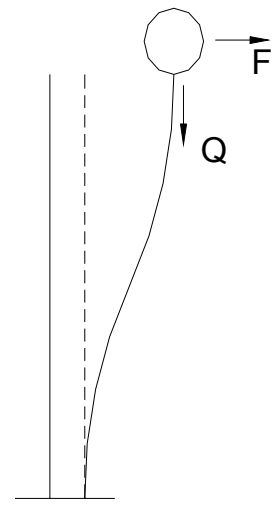
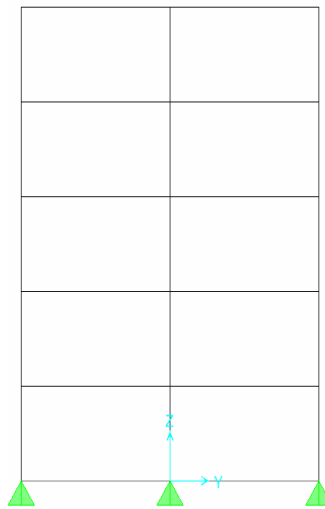
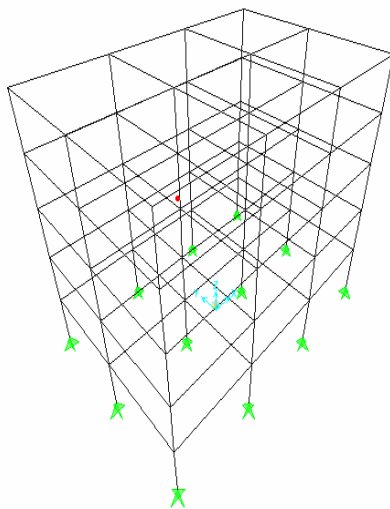
How to evaluate the damage of a city?
How to compute the damage on 500,000 housing?



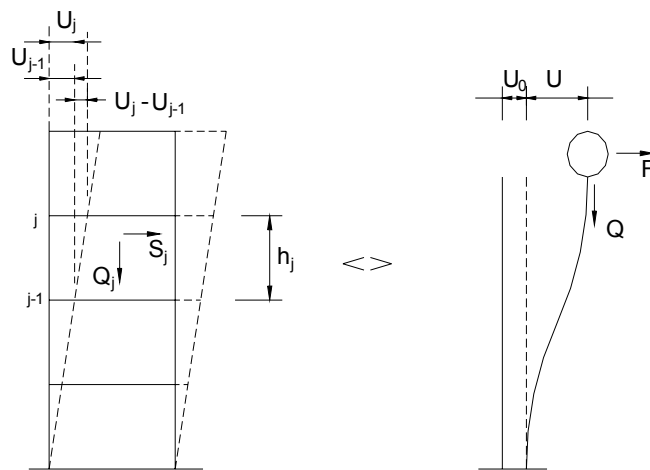
The first idea could be develop a serie of 3D models



**But, Can we make 500,000 models?
It is almost impossible**

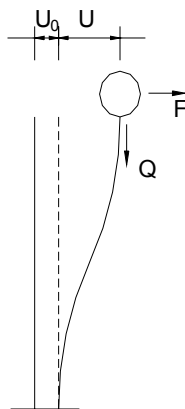


However we can make equivalent models from the actual Building using uncoupled equation of motion for the predominant mode and solve the system in aproximate way to diagnose the seismic response of the structure



Real Model

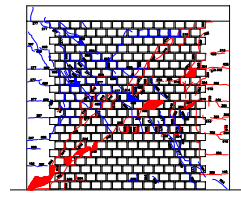
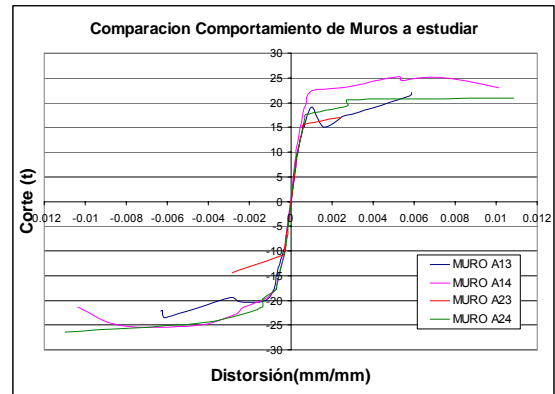
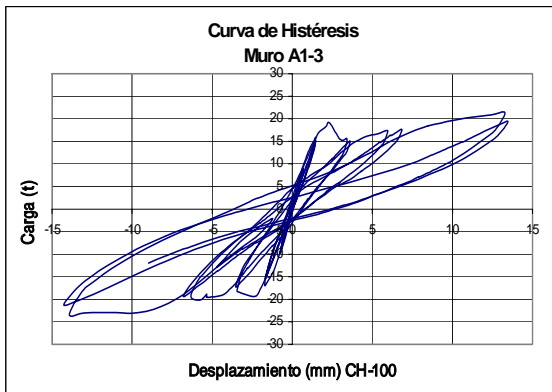
Equivalent Model



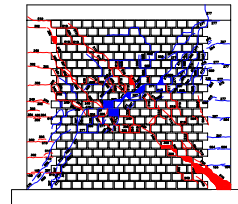
On the Equivalent model

1. The mass can be computed from the area of the building.
2. The stiffness of the building can be computed from the type of materials, geometry and amount of walls and columns.
3. The stiffness of a wall could be computed using a data base from full scale experimental test results.
4. Using an spring solver program the response for a deterministic structure can be computed.

TESTING AND GETTING THE BEHAVIOR CURVE



Muro A1-3
Distorsión 1/100
Cara Frontal



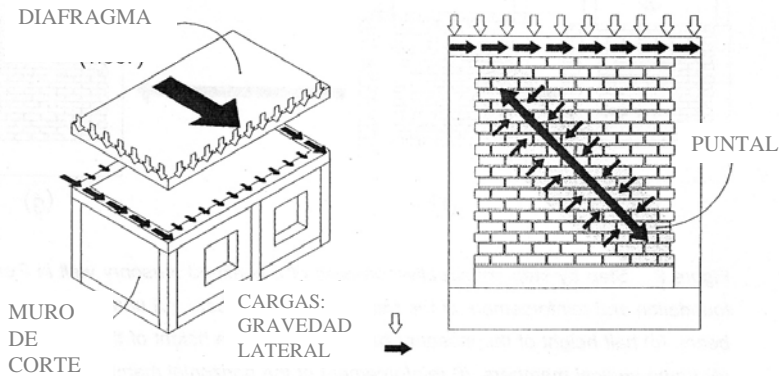
Muro A1-3
Distorsión 1/100
Cara Posterior

GETTING THE FAILURE PATTERN

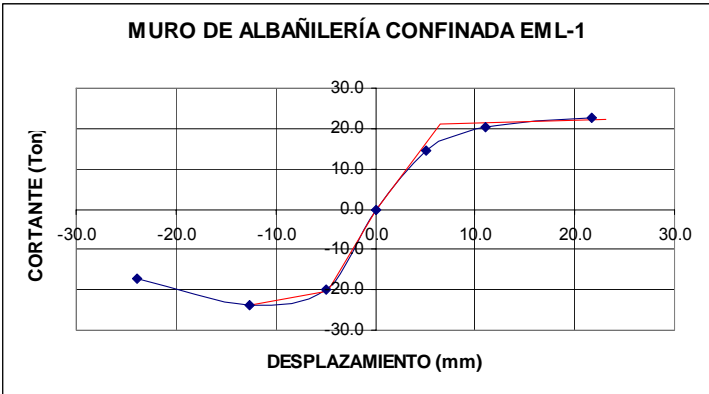
Types of walls studied experimentally and Considered for the diagnosis

Tipo	Junta	Aparejo	Refuerzo	Calidad	Espesor	Longitud	Altura	Esbeltez	K1(t/m)	K2(t/m)	Vmax (t)
1	Endentado	Cabeza	4 Φ 1/2	Artesanal	0.20	2.65	2.30	0.87	15517.33	471.97	17.74
2	Endentado	Soga	4 Φ 1/2	Artesanal	0.12	2.40	2.30	0.96	7768.40	425.90	9.50
3	Endentado	Soga	4 Φ 1/2	Industrial	0.13	2.40	2.30	0.96	14707.00	486.30	19.13
4	Endentado	Soga	4 Φ 3/8	Industrial	0.12	2.40	2.30	0.96	9433.03	488.27	17.40
5	Sin Endentar	Soga	4 Φ 3/8	Industrial	0.12	2.40	2.30	0.96	7752.50	737.50	16.65
6	Endentado	Soga	4 Φ 3/8	Industrial	0.12	1.80	2.40	1.33	3880.90	132.10	11.37
7	Endentado	Soga	4 Φ 3/8	Industrial	0.12	2.40	2.40	1.00	5310.55	237.20	17.40
8	Endentado	Soga	4 Φ 3/8	Industrial	0.12	3.60	2.40	0.67	7894.05	384.75	23.56
9	Endentado	Soga	4 Φ 3/8	Industrial	0.12	2.40	2.20	0.92	3932.40	45.69	22.41
10	Endentado	Soga	4 Φ 1/2	Industrial	0.12	2.40	2.20	0.92	4341.96	0.00	13.86
11	Endentado	Soga	4 Φ 3/8	Industrial	0.12	2.40	2.20	0.92	4482.20	147.89	22.15
12	Endentado	Soga	4 Φ 3/8	Industrial	0.12	2.40	2.20	0.92	5499.00	335.27	22.21
13	Endentado	Soga	4 Φ 3/8	Industrial	0.12	2.40	2.20	0.92	5017.57	57.65	21.07

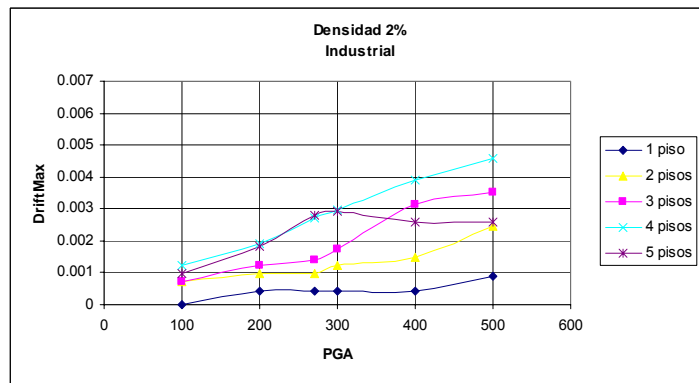
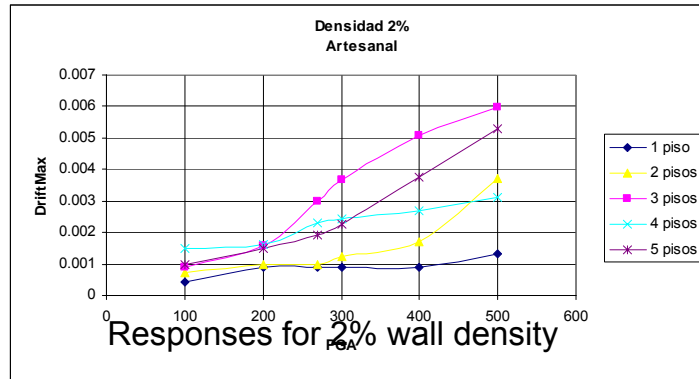
MODELLING WITH MACRO MODEL



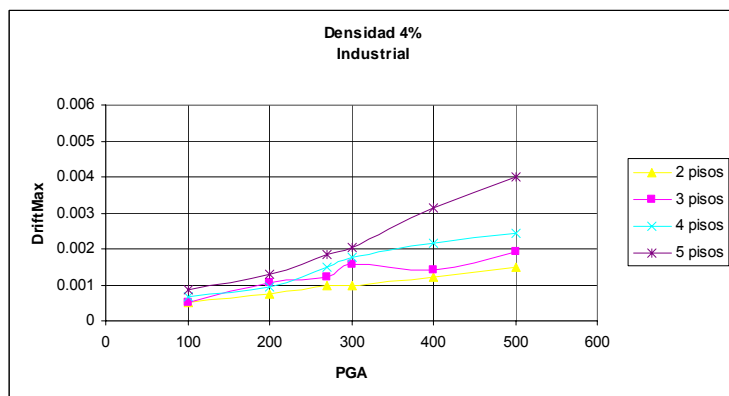
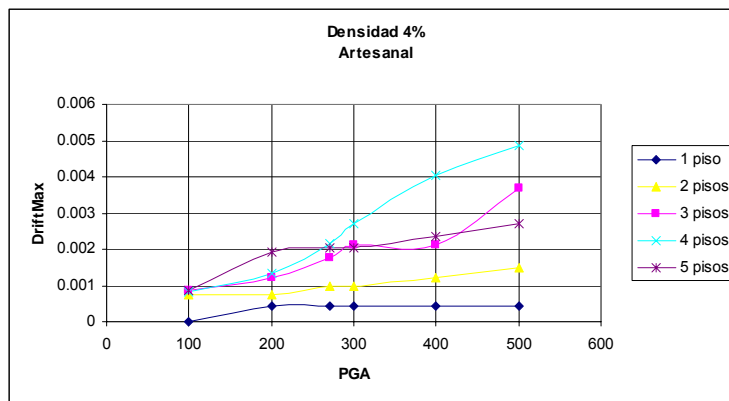
Bilinear model from testing wall



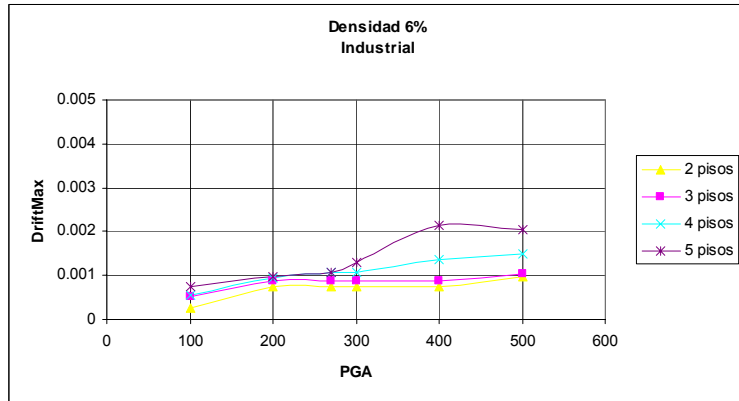
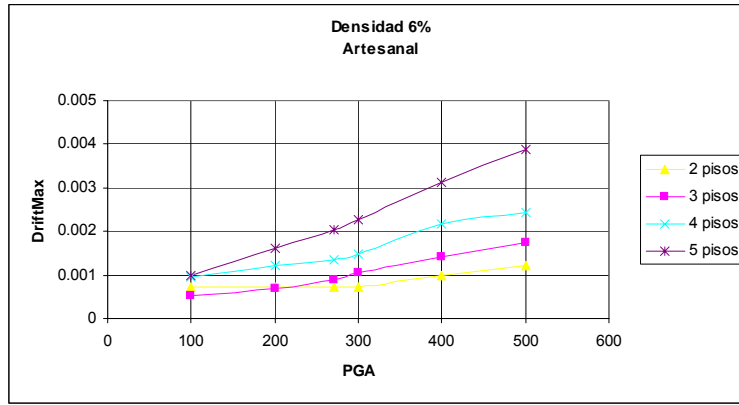
Responses for 2% wall density



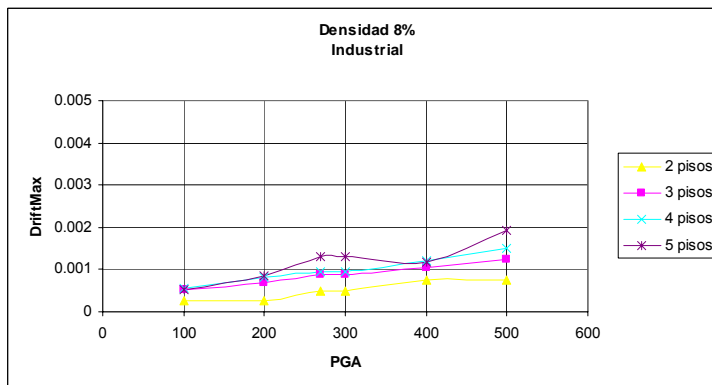
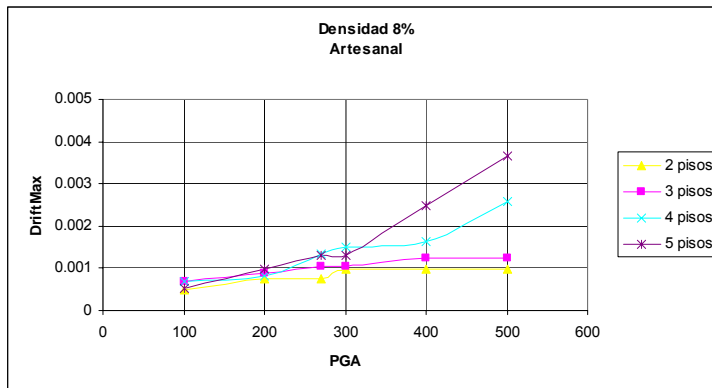
Responses for 4% wall density



Responses for 6% wall density



Responses for 8% wall density



Application on Survey



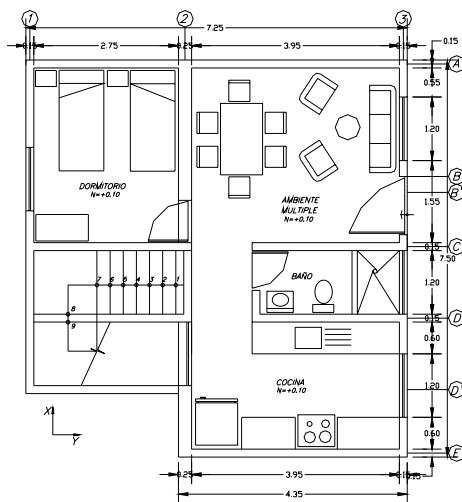
Important key data

- Story number
- Amount of walls
- Type of construction
- Material

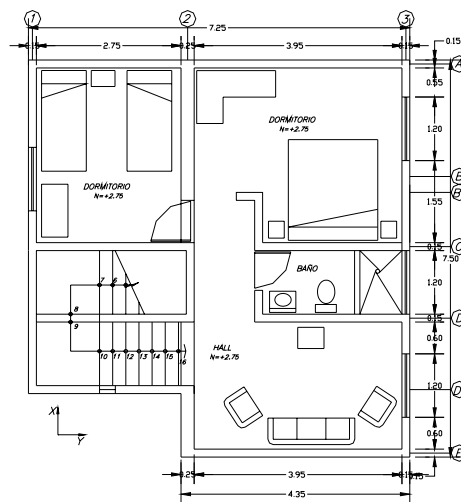
AGREEMENT CISMIDUNI - MUNICIPALITY OF LA MOLINA SEISMIC VULNERABILITY OF LA MOLINA DISTRICT		
LOCATION: BLOCK: _____ HOUSE: _____		
TYPE OF BUILDING:		SKETCH
One family house () Multifamily home () Home & business ()		
Commercial () Public () Private ()		
CHARACTERISTICS:		
Independent () Dpt. on building () Share ()		
Passage () Other _____		
OWNER:		
Own () Rental ()		
TIME FROM BUILT _____ Years		
NUMBER OF PERSONS: _____		
OCCUPATION		
Professional () Technician () Business () None ()		
AREA OF THE LAND _____ X _____ = _____ m ²		
AREA BUILT _____ X _____ = _____ m ²		
NUMBER OF STORIES _____ UNDERGROUND : YES () No _____		
STORY HEIGHT _____ NO ()		
TOTAL HEIGHT _____ HALF UNDERGROUND ()		
MAIN CONSTRUCTION MATERIAL :		
Adobe () Masonry () Reinforce Concrete () Other: _____		
ADOBRE	MASONRY	REINFORCE CONCRETE
FOOTING	FOOTING	FOOTING
Stone and mud () Stone and Cement ()	Beam without reinforce () Beam with reinforce ()	Isolate () Connected ()
Other: _____	Other: _____	Other: _____
STATE OF CONSERVATION	STATE OF CONSERVATION	STATE OF CONSERVATION
Good ()	Good ()	Good ()
Regular ()	Regular ()	Regular ()
Bad ()	Bad ()	Bad ()
No settlements ()	No settlements ()	No settlements ()
With settlements ()	With settlements ()	With settlements ()
ADOBRE	MASONRY	REINFORCE CONCRETE
ROOF SYSTEM	ROOF SYSTEM	ROOF SYSTEM
Wood () Can () Mud ()	Concrete Slab () Light slab ()	Concrete slab ()
Other: _____	Other: _____	Light slab ()
STATE OF WALLS	STATE OF WALLS	ELEMENTS :
GOOD ()	GOOD ()	WITHOUT CRACKS ()
REGULAR ()	REGULAR ()	COLUMNS ()
BAD ()	BAD ()	WITHOUT CRACKS ()
WITH COVERING ()	WITH COVERING ()	WITH CRACKS ()
WITHOUT COVERING ()	WITHOUT COVERING ()	COMENTARY: _____
COMENTARY: _____	COMENTARY: _____	COMENTARY: _____
REINFORCE ON WALLS	VERTICAL AND/OR HORIZONTAL REINFORCE	BEAMS :
YES ()	IN WALLS	WITHOUT CRACKS ()
NO ()	YES ()	WITH CRACKS ()
COMENTARY: _____	NO ()	COMENTARY: _____
	IF ANSWER IS POSITIVE FILL THE FOLLOWING:	CONCRETE WALLS:
	ONLY COLUMNS () ONLY BEAMS ()	WITHOUT CRACKS ()
	BEAMS AND () WITH REINFORCE ()	WITH CRACKS ()
	COLUMNS INSIDE THE BLOCKS	COMENTARY: _____
	COMENTARY: _____	COMENTARY: _____

FORMAT

VERIFYING THE PROPOSAL WITH A REAL BUILDING



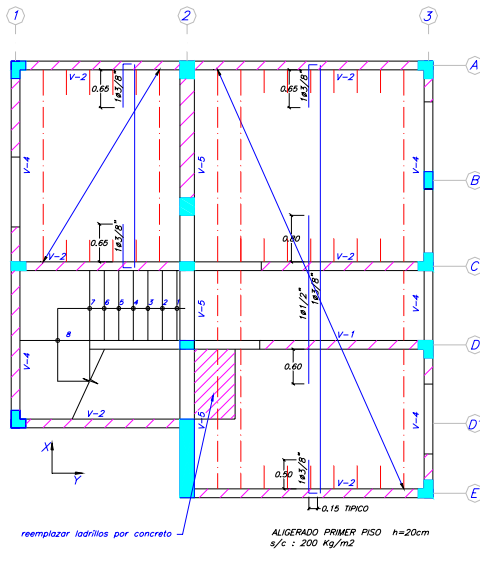
PLANTA PRIMER NIVEL



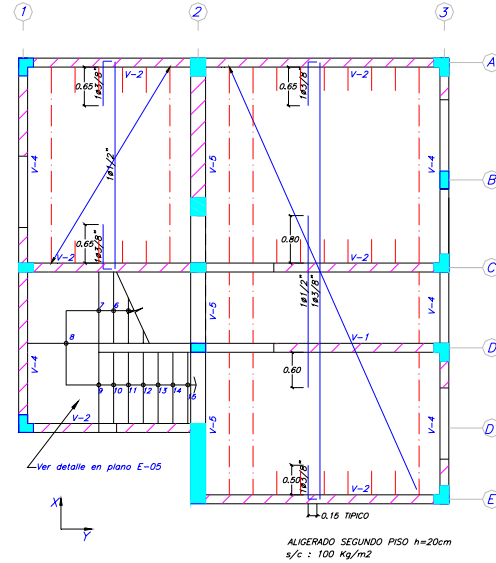
PLANTA SEGUNDO NIVEL

ARCHITECTURAL PLAN

STRUCTURAL PLAN OF THE BUILDING



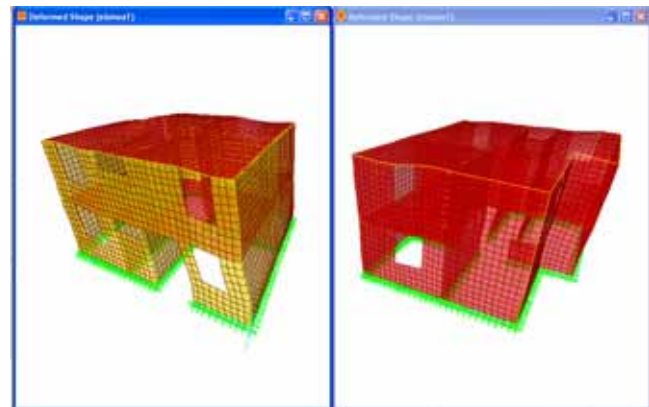
DMx= 4.%



DMy= 4.7%



MODELLING WITH FEM PROGRAM



FEM RESULTS

Table 10: Maximo Drift sismo moderado

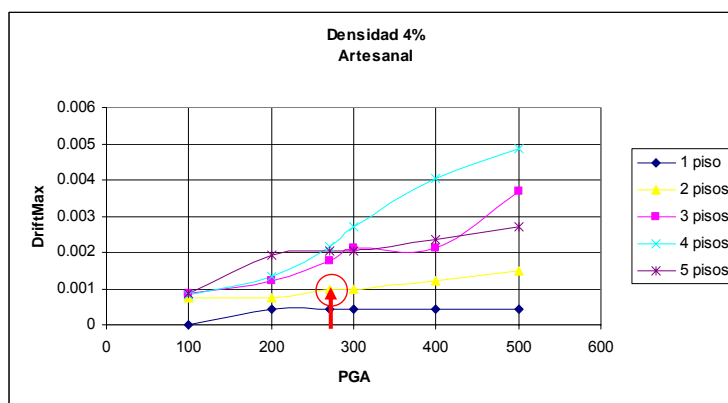
	Vx(kg)	Kx(kg/cm)	δx_i (cm)	Δx_i	
Story 2	18539.78	261401.9	0.319159874	0.001276639	<1/200
Story 1	27284.96	279239.6	0.439702385	0.001758810	<1/200

En nuestro ejemplo, para el sismo severo en la direccion X tenemos:

Table 11: Maximo Drift sismo severo

	Vx(kg)	Kx(kg/cm)	δx_i (cm)	Δx_i	
Piso2	18539.78	261401.9	0.319159874	0.001276639	<1/200
Piso1	27284.96	279239.6	0.439702385	0.001758810	<1/200

GETTING OUR PROPOSAL RESULTS



For DM_y=4% the diagnosis response is 0.00100

From SAP the diagnosis response is 0.00127

CONCLUSIONS

This report shows the first steps in the development of a method for evaluation of seismic response of buildings, both in urban or rural area. We consider our experimental background to shear the behavior curves and maximum threshold for each type of wall. However there are many influences we continue studying such as the influence of axial loads, overturning moments and other external factors. The authors consider this methodology a diagnosis proposal for screening evaluation. We want to provide in near future a second version of the method with the consideration of the drift thresholds proposed on [4] to evaluate the damage by the quake demand